



## Welded Joints Subject to Static Axial and direct shear loading:

CODE AWS structural welding code.

aim is to 'fuse' two materials into a single, homogeneous member.

AWS & ASTM standardize welding electrode Specs (strength, ductility, etc.)

e.g. E 60 XX      60  $\Rightarrow$   $S_{ut}$  at least 60 ksi.

E 70 XX      70  $\Rightarrow$   $S_{ut}$  " " 70 ksi

$S_y$  is about 12 ksi below tensile str.  
min. elongation bet. 17-25 %.

these properties apply to as-welded material.

Types of weld:

Butt weld



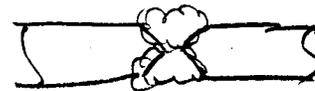
single V groove



single bevel

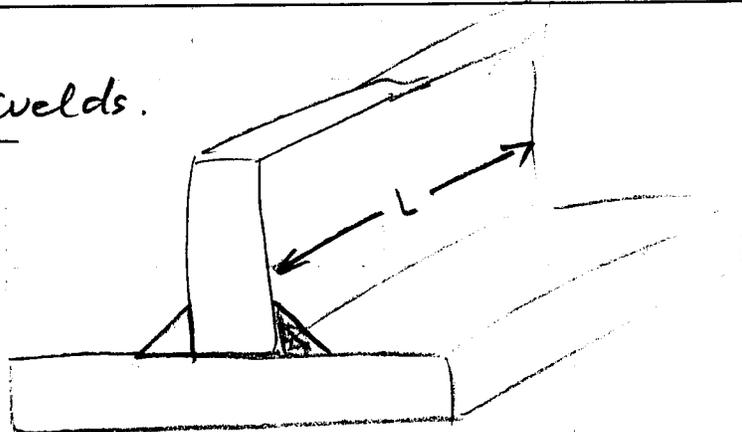


open square

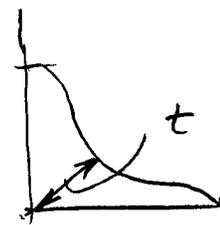
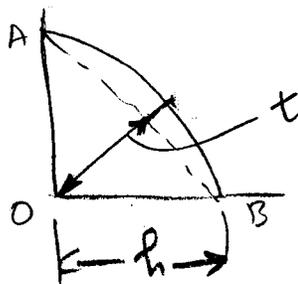


double V groove

## Fillet welds.



Convex  
bead.



(poor weld)  
Concave bead

"h"  $\Rightarrow$  size of the weld.

Significant stress:  
Conventional Engg Practice

Shear stress at the  
throat section.

t = shortest distance from  
O to AB

$$t = \frac{1}{\sqrt{2}} h$$

$$t = .707 h$$

$$(2t)^2 = 2h^2$$

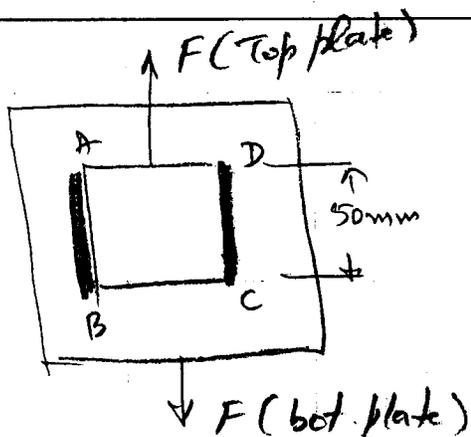
$$4t^2 = 2h^2$$

$$t^2 = \frac{1}{2} h^2$$

$$t = \frac{1}{\sqrt{2}} h$$

throat area =  $tL$

weld size h at least 3mm for plate < 6 mm thick  
and at least 15 mm for plates > 15 mm thick.



### EXAMPLE #1

Parallel-loaded Fillet weld.

Plates 12 mm thick of steel  $S_y = 350 \text{ MPa}$   
welded along AB, CD, each 50 mm long.

$$S_{y, \text{weld}} = 350 \text{ MPa.}$$

with a f.o.s of 3 (based on yield str.)

what static load  $F$  can be carried using a 6 mm weld? <sup>weld?</sup>  
leg

Assumption: weld fails in shear at throat.

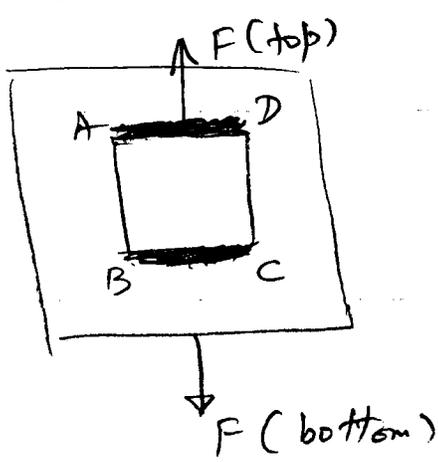
$$\begin{aligned} 1) \text{ total weld throat area} &= A = tL \\ &= (0.707)(6)(100) \\ &= 424 \text{ mm}^2 \end{aligned}$$

$$2) \text{ distortion energy theory: } S_{Sy} = 0.577 S_y = 203 \text{ MPa}$$

$$\begin{aligned} 3) \quad n \left( \frac{F}{A} \right) &= S_{Sy} & F &= \frac{S_{Sy} \cdot A}{n} \\ & & &= \frac{(203)(424)}{3} = 28,700 \text{ N} \\ & & &= 28.7 \text{ kN.} \end{aligned}$$

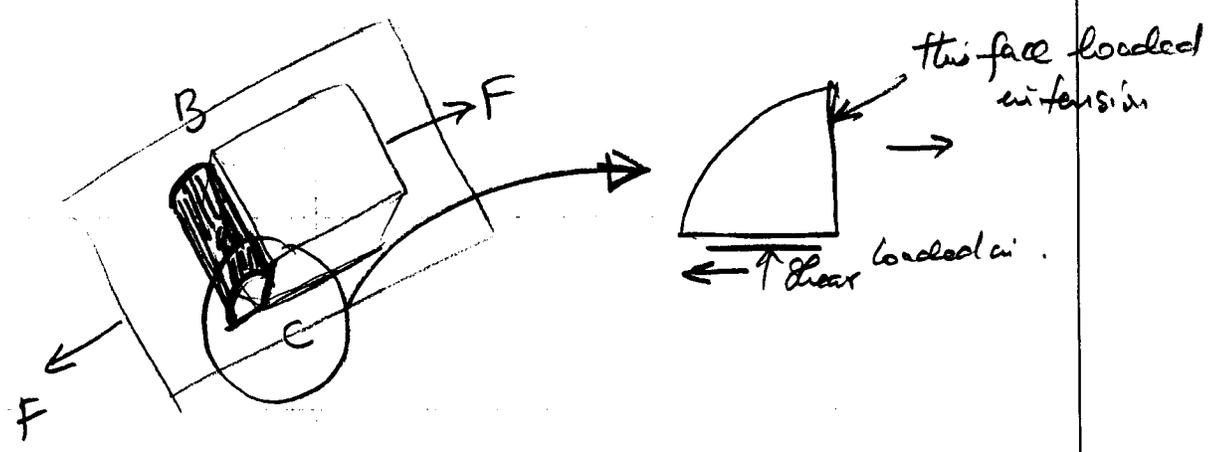
EXAMPLE #2.

Transversely loaded fillet weld



Suppose now sides AD and BC are welded. the weld is transversely loaded.

Again we assume that the weld fails at the throat, which carries the entire load in shear.



Assumption.

As the load F 'flow' thru the weld metal the loading has varying proportions of tension & shear. the critical section is assumed to be the minimum throat section, with area  $tL$ , which carries entire ~~the~~ load in shear ~~alone~~.

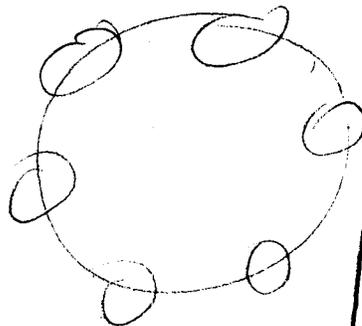
$$\frac{F}{A} = \frac{S_{34}}{n} \qquad F = 28.7 \text{ kN as before.}$$

(this solution is less rigorous than Ex 1. due to assumption made)

## Note on bolt spacing FOR PROJECTS

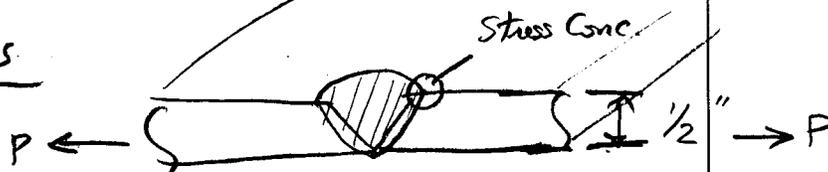
$$3 < \frac{\pi d_b}{N} < 6$$

wrench clearance                      effective Seal.



## Fatigue Considerations

( $K_f$  Tabulated).



butt-weld.

for fatigue, better to grind the weld flat to eliminate stress raiser.

## Example # 3



$P$  fluctuates between 5000 and 15000 lb.

Plates are  $\frac{1}{2}$  in. thick.

weld is not ground off. ;  $K_f = 1.2$

E 60 welding rod is used with  $S_u = 62$  ksi.

$n, f_{os}, = 2.5$

$S_y = 50$  ksi.

what length of weld  $L$  is required?

Assumptions:

- infinite life is required
- $k_c, ax = 0.85$ .
- ~~$k_f$~~  assume surface condition comparable to as-forged surface (Table 7-5, p 375)

$$K_{surf} = 39.8 (62)^{-0.995}$$

$$= 0.655$$

$$S_e' = 0.506 (62) = 31.4 \text{ ksi}$$

$$S_e = (31.4)(0.85)(0.655) = 17.5 \text{ ksi}$$

with a fatigue  $fos$  of 2.5, the tensile load varies from 12,500 to 37,500 lb.

weld area =  $15L$

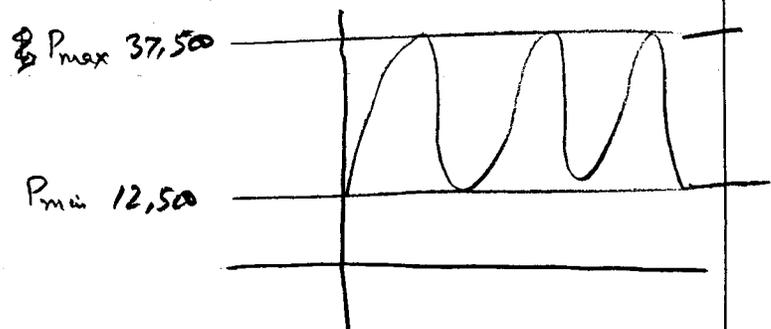
$$\sigma_m = K_f \frac{P_m}{A}$$

$$\sigma_a = K_f \frac{P_a}{A}$$

$$\sigma_m = \frac{60,000}{L}$$

$$\sigma_a = \frac{30,000}{L}$$

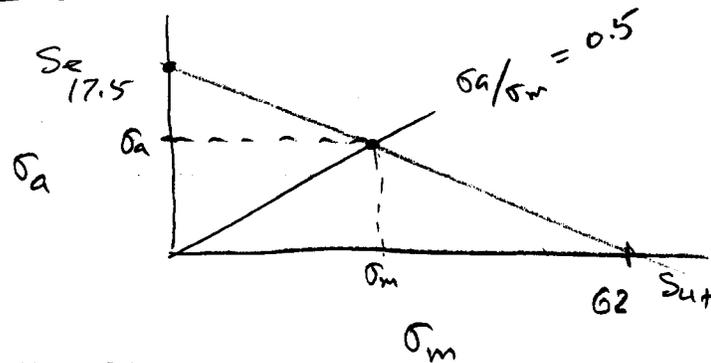
$$\sigma_a / \sigma_m = 0.5$$



$$P_m = \frac{1}{2} (P_{max} + P_{min}) = 25,000$$

$$P_a = \frac{1}{2} (P_{max} - P_{min}) = 12,500$$

Goodman :



$$\frac{\sigma_a}{S_e} + \frac{\sigma_m}{S_u} = 1.0$$

$$\frac{\sigma_a}{\sigma_m} = \frac{1}{2}$$

$$\sigma_m = 2 \sigma_a$$

$$\frac{\sigma_a}{S_e} + \frac{(2\sigma_a)}{S_u}$$

$$\frac{\sigma_a}{S_e} + \frac{2 \sigma_a}{S_u} = 1.0$$

$$\sigma_a S_u + 2 \sigma_a S_e = S_e S_u$$

$$\sigma_a (S_u + 2 S_e) = S_e S_u$$

$$\sigma_a = \frac{(17.5)(62)}{62 + (2 \times 17.5)}$$

$$= 11.18 \text{ ksi}$$

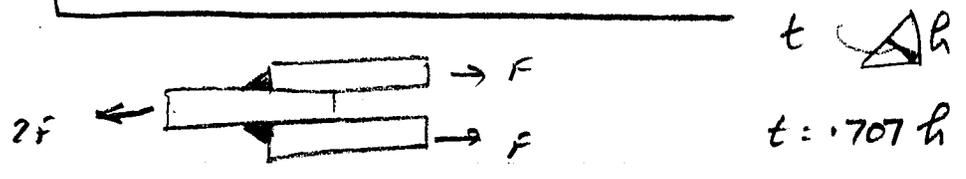
$$\sigma_a = \frac{30,000}{L} = 11.18 \text{ ksi} = 11,180 \frac{\text{lb}}{\text{in}^2}$$

$$L = \frac{30,000}{11,180} = \underline{2.68 \text{ in}}$$

L could be rounded off to 3.0 in.

— Ans.

Read 9.1, 9.2 (Butt & Fillet welds)



Shear stress at throat  $\tau = \frac{F}{tL} = \frac{S_{sy}}{n(fos)}$

Example #4

The 1018 steel strap has a 1000 lb, completely reversed load. Evaluate the fatigue strength of the weld for infinite life. for  $n_d = 3.0$  ( $n_d =$  design fos)

- (a) use the conventional ~~sa~~ factor-of-safety approach.
- (b) use ATSC code fatigue allowable stress method.

1018 Steel

Table E 20

$S_{ut} = 58$  ksi  
 $S_y = 32$  ksi.

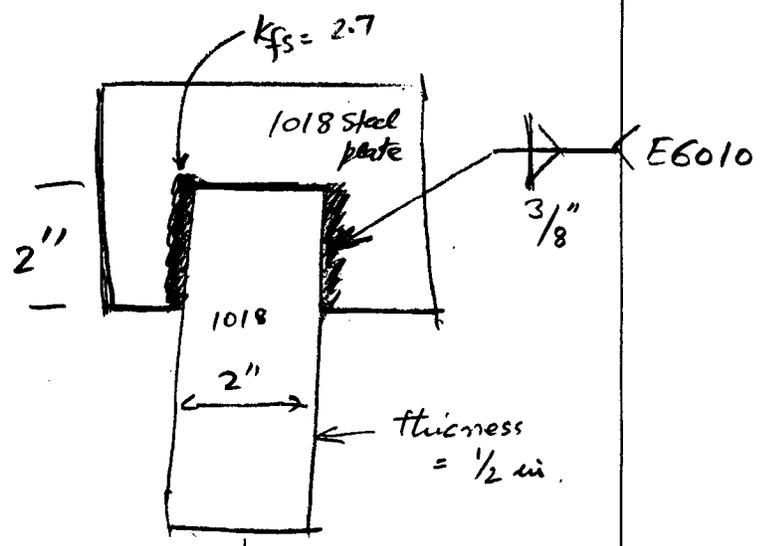
E 60 Electrode  
 (Text Table 9-4)

$S_{ut} = 62$  ksi.  
 $S_y = 50$  ksi.

$K_{fs} = 2.7$

Throat Area:

$A = (.707)(.375)(2 \times 2) = 1.061 \text{ in}^2$



1000 lb  
 fully reversed.

Analysis is based on weaker of the two materials.

surface factor (assume as-forged surface)

$$k_a = 39.8 (58)^{-1.995} = 0.7$$

Size factor:

for uniform shear stress on the throat, use  $k_b = 1$

Load factor: (for uniform shear, use  $k_c$  for torsion)

$$k_c = 0.59 \text{ (average)}$$

$$\text{OR } k_c = 0.328 (58)^{.125} = .545 \text{ (Table 7-7)}$$

Let us use  $k_c = .545$  in this example.

$$k_d \text{ (Temp)}, k_e \text{ (Misc)} = 1.0$$

Now

$$S_{se} = (.7)(1)(.545)(1)(1)(.506)(58) = 11.2 \text{ ksi}$$

$$F_a = 1000 \text{ lb}$$

$$F_m = 0$$

$$K_{fs} = 2.7$$

$$\tau_a = K_{fs} \frac{F_a}{A} = \frac{2.7(1000)}{1.061} = 2545 \text{ psi.}$$

$$n_f = \frac{S_{se}}{\tau_a} = \frac{11,200}{2545} = 4.4$$

Since  $n_f > n_{design} (3.0)$ , the weld is satisfactory for fatigue.

# Adequacy assessment of a weld

First decide: variables are:

decide first → weld pattern, Electrode, type of weld (eg fillet)

$l$  = length of weld | one of these may be  
 Leg size,  $h$  | design variable.

or, specify both  $l$  and  $h$ , and check to see if <sup>weld</sup> strength (static & fatigue are OK).

## Now for Project:

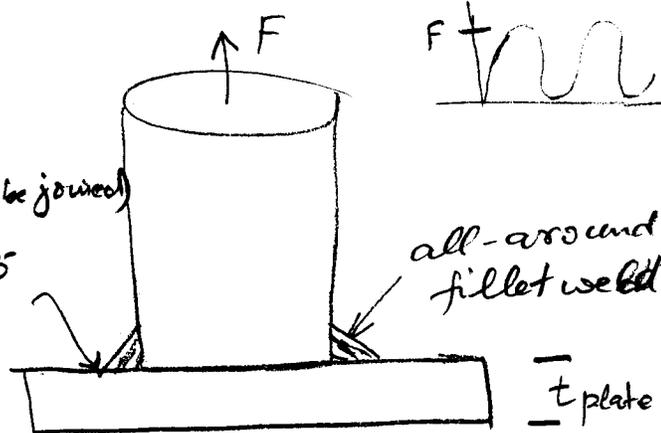
1) Pick electrode (stronger than ~~welded~~ materials to be joined)

60/70/80/  
90/100/120

Table 9-4

use.

$$K_{FS} = 1.5$$



all-around fillet weld

$t_{plate} = ?$

## Minimum fillet weld size, $h$ (Table 9-7)

Thickness of the <u>thicker</u> part	Min. weld size
up to 1/4", incl.	1/8
over 1/4" to 1/2"	3/16
1/2 to 3/4"	1/4
3/4 to 1 1/2"	5/16
1 1/2 to 2 1/4"	3/8
2 1/4 to 6"	1/2

F varies from 0 to F.

$$F_a = \frac{F}{2} \quad \rightarrow \quad F_m = \frac{F}{2}$$

Estimate  $S_{se}$  (weaker of the ~~for~~ weld, steel or plate)

$$\tau_a = K_{fs} \frac{F_a}{A}$$

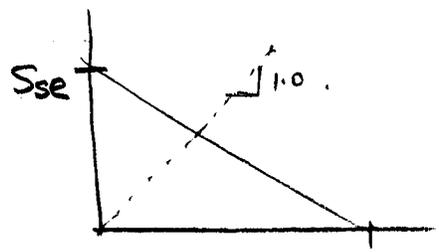
$$A = (\pi D_{shell}) (.707)(h)$$

$h = \text{weld leg.}$

$$\tau_m = K_{fs} \frac{F_m}{A}$$

Goodman:

$$\frac{\tau_a}{S_{se}} + \frac{\tau_m}{S_{su}} = \frac{1}{n_f}$$



$$S_{su} = .67 S_{ut}$$

find  $n_f$  and see how it compares to the design fof of 3.5

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